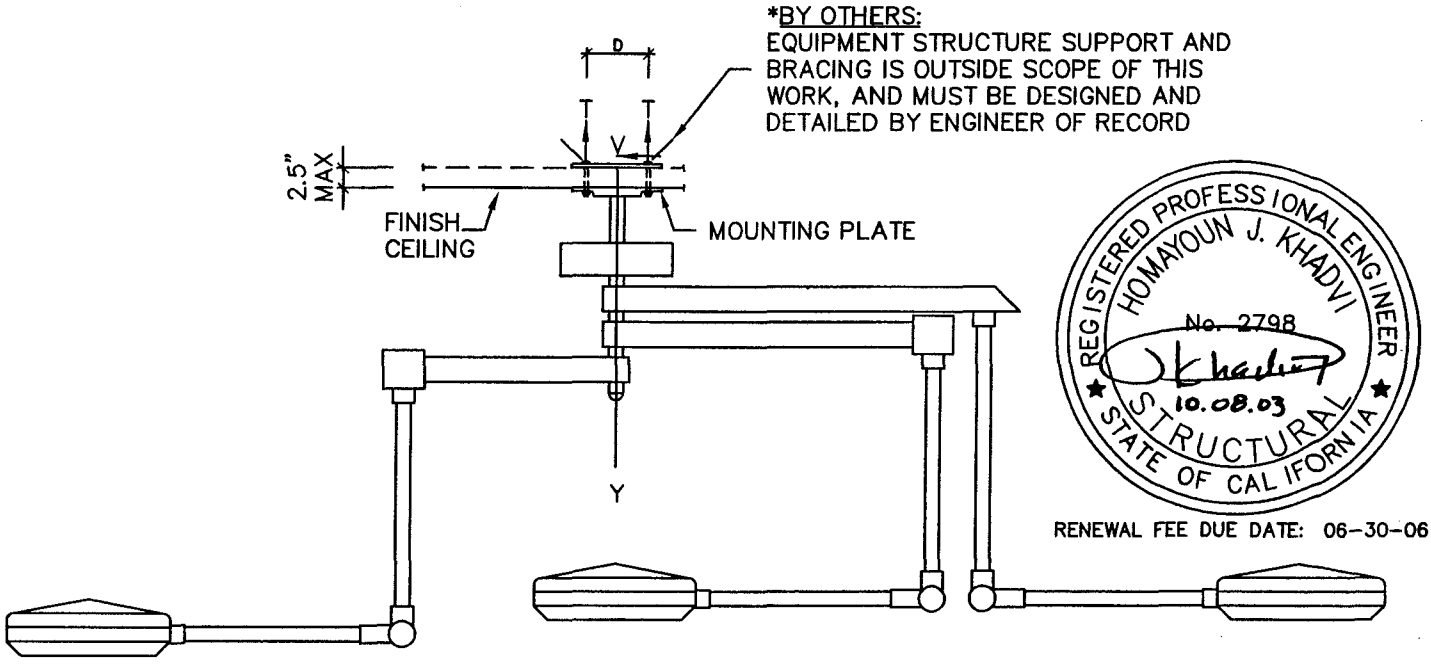
FIROUZI CONSULTING ENGINEER, INC.		
SKYTRON SURGICAL PRODUCTS	DES.	SHEET
STELLAR SERIES - ST 292323 / ST292323TV/ST29TV2323 FOR SEISMIC ZONE (4), SOIL PROFILE (Sd) NEAR SOURCE FACTOR = 1.5	FCE JOB No. DATE: 10-08-03	1 OF 4 SHEETS

SEISMIC ANCHORING BOLT DESIGN
ST292323/ST292323TV/ST29TV2323



ELEVATION

NOTES:

1. SCOPE OF WORK: DESIGN OF BOLTS CONNECTING MOUNTING PLATE TO STRUCTURE ONLY.
2. FORCES ARE DETERMINED PER 2001 CALIFORNIA BUILDING CODE – SECTION 1632A, (INCLUDING UP TO DATE REVISIONS) AND HAVE BEEN FACTORED TO REPRESENT WORKING DESIGN LOADS, NOT ULTIMATE
3. FORCES ARE MAXIMUMS AND OCCUR WHEN EQUIPMENT IS MOVED TO ITS MOST ECCENTRIC POSITION.
4. PROVIDE CEILING STRUCTURE DESIGNED AND DETAILS TO SUPPORT WEIGHTS AND FORCES SHOWN (BY ENGINEER OF RECORD FOR THE BUILDING)
5. ENGIINEER OF RECORD TO DESIGN, DETAIL AND VERIFY STRUCTURE AND/ OR EXISTING LIGHT SUPPORT TRACTS TO SUPPORT INDICATED LOADS
6. HORIZONTAL FORCES AND MOMENT MAY OCCUR IN ANY DIRECTION, ACTING AT THE TOP OF MOUNTING PLATE.

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STELLAR SERIES - ST 292323 / ST292323TV/ST29TV2323 FOR SEISMIC ZONE (4), SOIL PROFILE (Sd) NEAR SOURCE FACTOR = 1.5	FCE JOB No. DATE: 6-21-03	2 OF 4 SHEETS

DESIGN CRITERIA:

FORMULA 32A-1: $F_P = 4.0\ Ca*Ip*Wp$

TABLE 16A-Q : $Ca = 0.44*Na = 0.44*1.5 = 0.66$ (For zone 4 & S_D)

TABLE 16A-K : Ia = 1.5 (For essential facility)

$\therefore F_P = (4.0)(0.66)(1.5)Wp = 3.96\ Wp$ (For LRFD)

$F_P = 3.96Wp/1.4 = 2.83Wp$ (For ASD)

FORMULA 30A-1: $E = p*E_h + E_v$

$E_h = F_P$

p = 1.0 (FOR COMPONENT)

$E_v = (0.5)Ca*Ip*Wp$

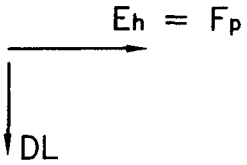
$= (0.5)(0.66)(1.5)Wp = 0.5Wp$ (For LRFD)

$= 0$ (For ASD)

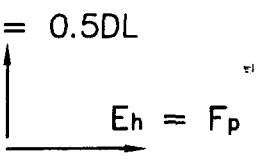
SECTION 1630A.11: $E_v = (0.7)Ca*I*Wp$

$= (0.7)(0.66)(1.5)/1.4 = 0.5Wp$ (For ASD) [NET UPLIFT FORCE]

LOAD COMBINATION CASE A



LOAD COMBINATION CASE B



'Y COMPARISION LOAD, COMBINATION A GOVERNS

FIROUZI CONSULTING ENGINEER, INC.

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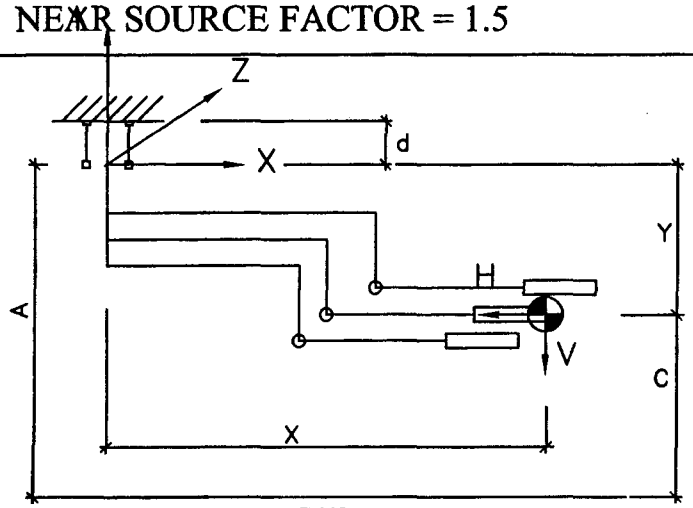
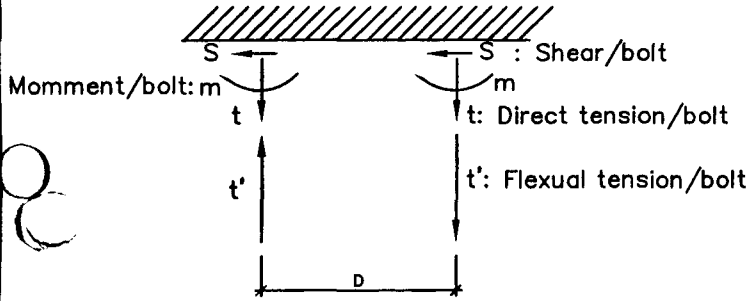
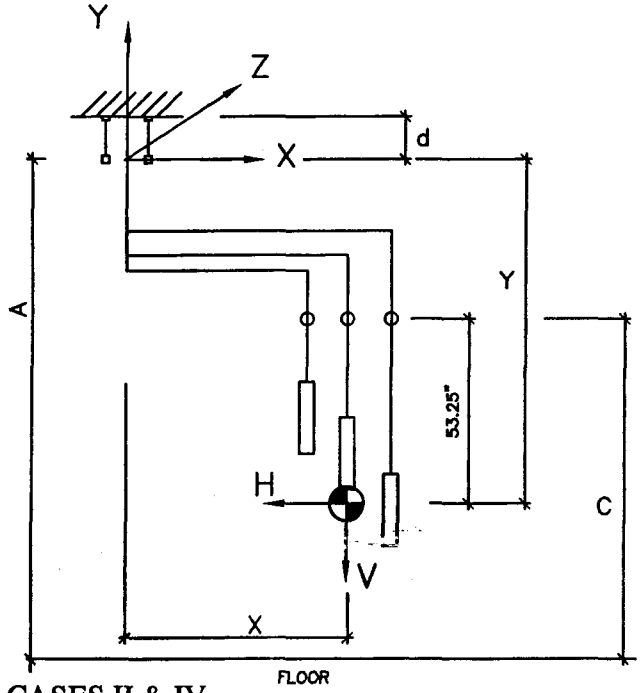
STELLAR SERIES - ST 292323 / ST292323TV/ST29TV2323 FOR SEISMIC ZONE (4), SOIL PROFILE (Sd) NEAR SOURCE FACTOR = 1.5	FCE JOB No.	3
	DATE: 10-08-03	OF 4 SHEETS



CASES I & III





CASES II & IV

A: MAX FLOOR TO MOUNTING PLATE DISTANCE
A = 10'-0" = 120"
C = 80"
Vd : Dead Load
Ve : Vertical Seismic Load
He : Horizontal Seismic Load

CASE I (FIXTURE AT HIGH POSITION)

d = 2.5 "
D = 9.5 "
Vd = 324 #
Y = A - C = 120 - 80 = 40 "
X = (104 + 86 + 70.5)/ 3 = 86.8 "
Ve = 0.50 x 324 = 160.4 #
He = 2.83 x 324 = 916.5 #
S = 916.4571429 / 4 = 229 # per bolt
t = (324 + 160.38)/ 4 = 121 # per bolt
Total Mzz = (324 + 160.3)x 86.8 + 916.4 x 40 = 78719 "#
t' = 78719 /(9.5 x 2) = 4143 # per bolt
m = 229.1 x 2.5 = 573 "# per bolt

CASE II (FIXTURE AT HIGH POSITION)

d = 2.5 "
D = 9.5 "
Vd = 324 #
Y = A - C + 53 = 93 "
X = (50.5 + 43 + 35.2)/ 3 = 42.9 "
Ve = 0.50 x 324 = 160.4 #
He = 2.83 x 324 = 916.5 #
S = 916.4 / 4 = 229.1 # per bolt
t = (324 + 160.4)/ 4 = 121 # per bolt
Total Mzz = (324 + 160.4)x 42.9 + 916.5 x 93 = 106010 "#
t' = 106010 /(9.5 x 2) = 5579 # per bolt
m = 229.1 x 2.5 = 572.7 "# per bolt

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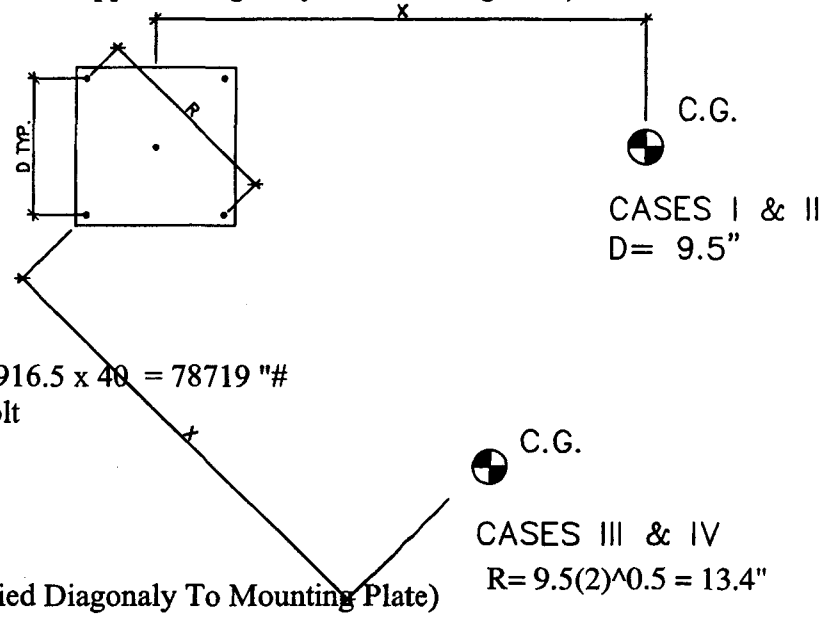
CASE III (Same As CASE I, Lateral Forces Applied Diagonaly To Mounting Plate)



d = 2.5 "
R = 13.4 "
Vd = 324 #
Y = 40 "
X = 86.8 "
Ve = 0.50 x 324 = 160.4 #
He = 2.83 x 324 = 916.5 #
S = 916.5 / 4 = 229.1 # per bolt
t = (324 + 160.4)/ 4 = 121 # per bolt
Total Mzz = (324 + 160.4)x 86.8 + 916.5 x 40 = 78719 "#
t' = 78719 /(13.4 x 1) = 5859 # per bolt
m = 229.1 x 2.5 = 572.8 "# per bolt

CASE IV
(Same As CASE II, Lateral Forces Applied Diagonaly To Mounting Plate)

d = 2.5 "
R = 13.4 "
Vd = 324 #
Y = 93 "
X = 42.9 "
Ve = 0.50 x 324 = 160.4 #
He = 2.83 x 324 = 916.5 #
S = 916.5 / 4 = 229.1 # per bolt
t = (324 + 160.4)/ 4 = 121.1 # per bolt
Total Mzz = (324 + 160.4)x 42.9 + 916.5 x 93 = 106010 "#
t' = 106010 /(13.4 x 1) = 7891 # per bolt GOVERNS
m = 229.1 x 2.5 = 572.8 "# per bolt

CHECK 3/4" DIA. A490 BOLTS:

ALLOWABLE TENSION:23,900#
ALLOWABLE SHEAR: 9,280#

S=3.14*d^3/32=3.14x(0.75)^3/32=0.04"3
fb=572.8 /0.04=13837PSI
Fb=0.75x36000=27000PSI

fv/Fv + ft/Ft + fb/Fb =0.02+0.33+0.51=0.87<1.0 OK

USE 3/4" DIA. A490 BOLTS